

FACULTY OF MECHATRONICS,  
INFORMATICS AND INTERDISCIPLINARY  
STUDIES TUL



# Poroelasticity: Theory, numerical solution and applications

SNA'23, Ostrava, 23 - 27 January 2023

**Jan Stebel**

Technical University of Liberec, Czech Republic

jan.stebel@tul.cz

Collaborators: J. Březina, P. Exner, M. Špetlík (TUL), J. Kružík, D. Horák (IGN)

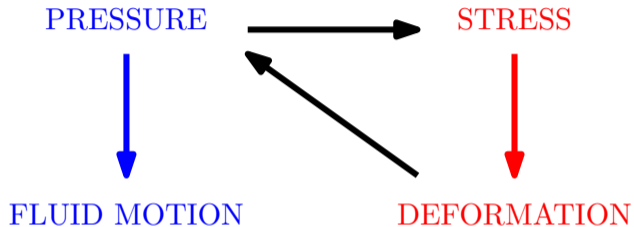
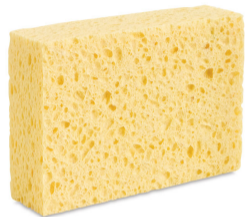
# Outline of presentation

- 1 Mathematical modelling of poroelasticity
- 2 Well-posedness
- 3 Approximation
- 4 Iterative splitting
- 5 Applications in rock hydro-mechanics

# Mathematical modelling

# What is poroelasticity I

poroelasticity = porous media flow + elasticity

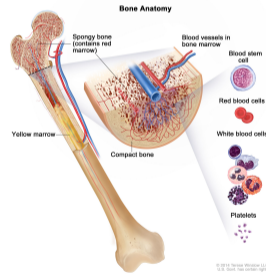
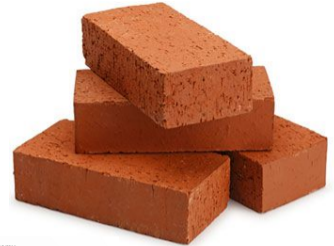
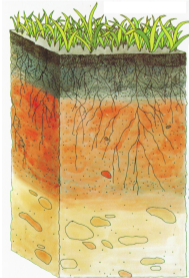




# What is poroelasticity II

Examples of deformable porous media

- soils, rocks
- biological tissues
- building materials



## Overview of literature

- K. Terzaghi (1923, 1925)
- M. A. Biot (1941)
- F. Gassmann (1951)
- E. Detournay and A. H.-D. Cheng (1993)
- A. Verruijt (2016)



K. Terzaghi  
(1883-1963)



M. A. Biot  
(1905-1985)

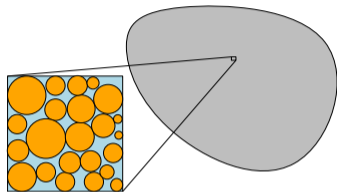
# Derivation of Biot model

## General assumptions

- macroscopically homogeneous material
- 2 components: solid skeleton + fluid filled pores
- linearly elastic solid skeleton
- interconnected pores

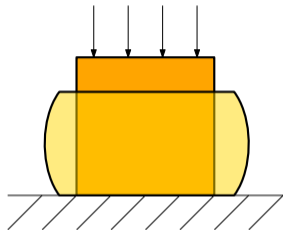
porosity:

$$\phi = \frac{V_f}{V} = 1 - \frac{V_s}{V}$$



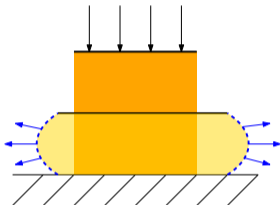
# Derivation of Biot model

## Drained and undrained deformations



Undrained deformation:

- fluid is entrapped in the domain
- stress is carried by
  - compression of fluid
  - compression of solid particles
  - rearrangement of solid skeleton



Drained deformation:

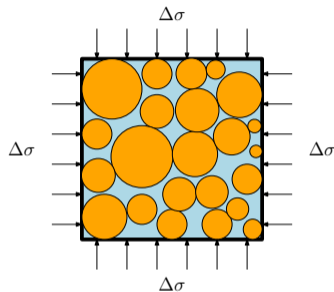
- fluid can flow in/out
- stress is carried only by solid particles and skeleton
- consolidation takes time due to low permeability

# Derivation of Biot model

## Undrained compression test

- hydraulically isolated sample
- applied stress  $\Delta\sigma = \underbrace{\Delta p}_{\text{pore pressure}} + \underbrace{(\Delta\sigma - \Delta p)}_{\text{effective stress}}$
- volume change of porous medium:

$$\begin{aligned}\frac{\Delta V}{V} &= \underbrace{-C_s \Delta p}_{\text{deformation due to pore pressure}} - \underbrace{C_m (\Delta\sigma - \Delta p)}_{\text{deformation (rearrangement of particles) at constant pore pressure}} \\ &= \underbrace{-\phi C_f \Delta p}_{\text{deformation of fluid}} - \underbrace{(1 - \phi) C_s \left( \Delta p + \frac{\Delta\sigma - \Delta p}{1 - \phi} \right)}_{\text{deformation of solid particles}}\end{aligned}$$



compressibilities:

$C_f$  - fluid

$C_s$  - solid particles

$C_m$  - porous medium

# Derivation of Biot model

## Undrained compression test

Skempton pore pressure coefficient:

$$B := \frac{\Delta p}{\Delta \sigma} = \frac{C_m - C_s}{(C_m - C_s) + \phi(C_f - C_s)} < 1 \quad \text{if } C_f, C_s > 0$$

Decomposition of total stress tensor into **effective stress** and **pore pressure**:

$$\sigma = \sigma' - \alpha p \mathbf{I}, \quad \alpha = \text{Biot-Willis coefficient}$$

Sign convention: compressive stress negative

For isotropic media:

$$\frac{\Delta V}{V} = \varepsilon = C_m \Delta \sigma' = C_m (\Delta \sigma + \alpha \Delta p) \Rightarrow \alpha = 1 - \frac{C_s}{C_m}$$

# Derivation of Biot model

## Balance laws

Assumption: Time scale of mechanics  $\ll$  time scale of porous media flow

Equilibrium equations for total stress:

$$-\operatorname{div} \boldsymbol{\sigma} = \boxed{-\operatorname{div} \boldsymbol{\sigma}' + \alpha \nabla p = \mathbf{f}}$$

Hooke's law:

$$\boldsymbol{\sigma}' = \mathbf{C} \boldsymbol{\varepsilon}(\mathbf{u}) \quad \Leftrightarrow \quad \boldsymbol{\varepsilon}(\mathbf{u}) = \mathbf{C}^{-1} \boldsymbol{\sigma}'$$

Isotropic media:

$$\mathbf{C} \boldsymbol{\varepsilon} = 2G \boldsymbol{\varepsilon} + \left(K - \frac{2}{3}G\right) (\operatorname{tr} \boldsymbol{\varepsilon}) \mathbf{I}, \quad K = \frac{1}{C_m}$$

$K$ ...bulk modulus

$G$ ...shear modulus

# Derivation of Biot model

## Balance laws

Conservation of mass:

$$\partial_t (\phi \rho_f) + \operatorname{div}(\phi \rho_f \mathbf{v}) = 0 \quad \partial_t ((1 - \phi) \rho_s) + \operatorname{div}((1 - \phi) \rho_s \mathbf{w}) = 0$$

Density of fluid and solid:

$$\rho_f(p) = \rho_{f0} \exp(C_f p) \quad \rho_s(p, \sigma) = \rho_{s0} \exp\left(\frac{C_s}{1 - \phi} (\phi p - \sigma)\right)$$

Conservation of mass for porous medium:

$$\operatorname{div} \mathbf{w} + \operatorname{div}(\phi(\mathbf{v} - \mathbf{w})) + \phi(C_f - C_s) \partial_t p - C_s \partial_t \sigma = 0$$

$$\mathbf{q} := \phi(\mathbf{v} - \mathbf{w}) \quad (\text{specific discharge})$$

$$\sigma := \sigma' - \alpha p = \operatorname{div} \mathbf{u} / C_m - \alpha p \quad (\text{isotropic stress})$$

$$S := \phi C_f + (\alpha - \phi) C_s \quad (\text{storativity})$$



# Derivation of Biot model

## Balance laws

Conservation of mass of porous medium (storage equation):

$$\alpha \partial_t \operatorname{div} \mathbf{u} + S \partial_t p = -\operatorname{div} \mathbf{q}$$

Darcy's law:

$$\mathbf{q} = -\frac{\kappa}{\mu} \nabla \left( p - \underbrace{\rho_f g e}_{\text{hydrostatic pressure}} \right) = -k \nabla \left( \underbrace{\frac{p}{\rho_f g} - e}_{\text{piezometric head}} \right)$$

$\kappa$ ...permeability (generally tensor)

$\mu$ ...fluid viscosity

$e$ ...elevation

$k$ ...hydraulic conductivity (generally tensor)

$g$ ...gravitational acceleration

## Biot system

Assuming  $\rho_f g = 1$ , the poroelasticity equations read:

$$\left. \begin{aligned} -\operatorname{div}(\mathbf{C}\boldsymbol{\varepsilon}(\mathbf{u})) + \alpha\nabla p &= \mathbf{f} \\ \partial_t(Sp + \alpha\operatorname{div}\mathbf{u}) - \operatorname{div}(k\nabla p) &= b \end{aligned} \right\} \text{ in } (0, T) \times \Omega$$

- Coupled linear elliptic-parabolic system with unknowns  $(\mathbf{u}, p)$
- Initial condition - only for flow:

$$p(0, \cdot) = p_0 \text{ in } \Omega$$

- Boundary conditions:

$$\begin{array}{ll} \text{Flow:} & p = p_D \text{ on } (0, T) \times \Gamma_{Df} \quad \mathbf{q} \cdot \mathbf{n} = q_N \text{ on } (0, T) \times \Gamma_{Nf} \\ \text{Mechanics:} & \mathbf{u} = \mathbf{u}_D \text{ on } (0, T) \times \Gamma_{Dm} \quad \boldsymbol{\sigma}'\mathbf{n} = \mathbf{t}_N \text{ on } (0, T) \times \Gamma_{Nm} \end{array}$$

# Generalizations and departures from poroelasticity

- Dynamic elasticity...hyperbolic-parabolic system:

$$\underbrace{\rho \partial_{tt}^2 \mathbf{u}}_{\text{inertial term}} + \underbrace{\lambda^* \nabla \partial_t \operatorname{div} \mathbf{u}}_{\text{secondary consolidation term}} - \operatorname{div}(\mathbf{C} \boldsymbol{\varepsilon}(\mathbf{u})) + \alpha \nabla p = \mathbf{f}$$

- Initial (reference) stress - important for nonlinear problems:

$$\boldsymbol{\sigma} = (\boldsymbol{\sigma}' - \boldsymbol{\sigma}_0) - \alpha p \mathbf{l}$$

- Hydro-mechanical parameter coupling, e.g.  $k = k(\mathbf{u}, \boldsymbol{\sigma})$
- Unsaturated flow
- Non-linear mechanics: plasticity, damage, fracture mechanics
- Thermo-poro-elasticity
- ...

# Well-posedness

# Biot system

## A priori estimate

- For simplicity we assume homogeneous b.c.:  $p = 0$ ,  $\mathbf{u} = \mathbf{0}$  on  $(0, T) \times \partial\Omega$
- Differentiate elasticity equation w.r. to time, multiply by  $\partial_t \mathbf{u}$ , and integrate:

$$\begin{aligned} & \int_{\Omega} \partial_t \mathbf{u} \partial_t (-\operatorname{div}(\mathbf{C}\boldsymbol{\varepsilon}(\mathbf{u})) + \alpha \nabla p) = \\ & = \int_{\Omega} \mathbf{C}\boldsymbol{\varepsilon}(\partial_t \mathbf{u}) : \boldsymbol{\varepsilon}(\partial_t \mathbf{u}) - \alpha \int_{\Omega} \partial_t p \operatorname{div} \partial_t \mathbf{u} = \int_{\Omega} \partial_t \mathbf{u} \cdot \partial_t \mathbf{f} \end{aligned}$$

- Multiply storage (flow) equation by  $\partial_t p$  and integrate:

$$\begin{aligned} & \int_{\Omega} \partial_t p (\partial_t (Sp + \alpha \operatorname{div} \mathbf{u}) - \operatorname{div}(k \nabla p)) = \\ & = S \int_{\Omega} |\partial_t p|^2 + \alpha \int_{\Omega} \partial_t p \operatorname{div} \partial_t \mathbf{u} + k \int_{\Omega} \partial_t \nabla p \cdot \nabla p = \int_{\Omega} b \partial_t p \end{aligned}$$

- Sum both equations...

# Biot system

## A priori estimate

- Sum of both equations:

$$\int_{\Omega} \mathbf{C}\varepsilon(\partial_t \mathbf{u}) : \varepsilon(\partial_t \mathbf{u}) + S \int_{\Omega} |\partial_t p|^2 + \frac{k}{2} \frac{d}{dt} \int_{\Omega} |\nabla p|^2 = \int_{\Omega} \partial_t \mathbf{u} \cdot \partial_t \mathbf{f} + \int_{\Omega} b \partial_t p$$

- Integrate w.r. to time:

$$\begin{aligned} \int_0^\tau \int_{\Omega} \mathbf{C}\varepsilon(\partial_t \mathbf{u}) : \varepsilon(\partial_t \mathbf{u}) + S \int_0^\tau \int_{\Omega} |\partial_t p|^2 + \frac{k}{2} \int_{\Omega} |\nabla p|^2(\tau) \\ = \int_0^\tau \int_{\Omega} (\partial_t \mathbf{u} \cdot \partial_t \mathbf{f} + b \partial_t p) + \frac{k}{2} \int_{\Omega} |\nabla p_0|^2 \end{aligned}$$

- If  $\mathbf{C}\varepsilon : \varepsilon \geq 2G|\varepsilon|^2$ ,  $S, k > 0$  then there is a constant  $C = C(G, S, k) > 0$ :

$$\int_0^T \left( \|\nabla \partial_t \mathbf{u}\|_2^2 + \|\partial_t p\|_2^2 \right) + \sup_{\tau \in (0, T)} \|\nabla p\|_2^2(\tau) \leq C \int_0^T \left( \|\partial_t \mathbf{f}\|_2^2 + \|b\|_2^2 \right) + \|\nabla p_0\|_2^2$$

# Biot system

## Weak solution

### Weak formulation of Biot problem (B)

Find  $\mathbf{u} \in H^1(0, T; \mathbf{H}_0^1(\Omega))$ ,  $p \in L^\infty(0, T; H_0^1(\Omega)) \cap H^1(0, T; L^2(\Omega))$  s.t.

- $p(0, \cdot) = p_0$  in  $\Omega$ ;
- $\forall \mathbf{v} \in \mathbf{H}_0^1(\Omega)$  and a.e.  $t \in (0, T)$ :

$$\int_{\Omega} [\mathbf{C}\boldsymbol{\varepsilon}(\mathbf{u}(t)) : \boldsymbol{\varepsilon}(\mathbf{v}) - \alpha p(t) \operatorname{div} \mathbf{v}] = \int_{\Omega} \mathbf{f}(t) \cdot \mathbf{v};$$

- $\forall q \in H_0^1(\Omega)$  and a.e.  $t \in (0, T)$ :

$$\int_{\Omega} [\partial_t (S p(t) + \alpha \operatorname{div} \mathbf{u}(t)) q + k \nabla p(t) \cdot \nabla q] = \int_{\Omega} b(t) q.$$

# Biot system

## Existence of weak solutions

Assumptions:

- $\forall \boldsymbol{\varepsilon} : \mathbf{C}\boldsymbol{\varepsilon} : \boldsymbol{\varepsilon} \geq 2G|\boldsymbol{\varepsilon}|^2 + (K - \frac{2}{3}G)|\text{tr } \boldsymbol{\varepsilon}|^2$
- $S, k > 0$
- $p_0 \in H_0^1(\Omega), \mathbf{f} \in H^1(0, T; \mathbf{L}^2(\Omega)), b \in L^2(0, T; L^2(\Omega))$

## Theorem

*Under the above assumptions, problem (B) has a unique solution.*

References:

- A. Ženíšek (1984): weak solutions,  $S = 0$
- R. E. Schowalter (2000): strong and weak solutions, quasistatic/dynamic case



# Approximation

# Approximation of two-field formulation of Biot system I

**Two-field (primal) formulation**  $(\mathbf{u}, p)$ :

$$\begin{aligned} -\operatorname{div}(\mathbf{C}\boldsymbol{\varepsilon}(\mathbf{u})) + \alpha\nabla p &= \mathbf{f} \\ \partial_t (S\rho + \alpha \operatorname{div} \mathbf{u}) - \operatorname{div}(k\nabla p) &= b \end{aligned}$$

Temporal semidiscretization (e.g. implicit Euler's method with equidistant timestepping,  $\Delta t = T/N$ ,  $t_i = i\Delta t$ ,  $i = 1, \dots, N$ ):

$$\begin{aligned} (\mathbf{C}\boldsymbol{\varepsilon}(\mathbf{u}(t_i)), \boldsymbol{\varepsilon}(\mathbf{v})) - (\alpha p(t_i), \operatorname{div} \mathbf{v}) &= (\mathbf{f}(t_i), \mathbf{v}) \\ (\alpha \operatorname{div} \mathbf{u}(t_i), q) + (S\rho(t_i), q) + \Delta t(k\nabla p(t_i), \nabla q) &= (\Delta t b(t_i) + S\rho(t_{i-1}) + \alpha \operatorname{div} \mathbf{u}(t_{i-1}), q) \end{aligned}$$

In operator form:

$$\begin{bmatrix} \mathcal{A} & -\mathcal{B}^\top \\ \mathcal{B} & \mathcal{C} \end{bmatrix} \begin{bmatrix} \mathbf{u}(t_i) \\ p(t_i) \end{bmatrix} = \begin{bmatrix} \mathbf{f}_i \\ b_i \end{bmatrix}$$

## Approximation of two-field formulation of Biot system II

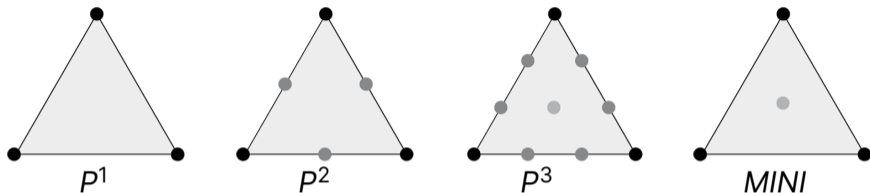
- Skew-symmetric saddle-point structure;
- For small  $S$  and  $\Delta t k$ ,  $\mathcal{C} \approx 0 \dots$  Biot  $\approx$  Stokes:

$$\begin{bmatrix} \mathcal{A} & -\mathcal{B}^\top \\ \mathcal{B} & 0 \end{bmatrix} \begin{bmatrix} \mathbf{u}(t_i) \\ p(t_i) \end{bmatrix} = \begin{bmatrix} \mathbf{f}_i \\ b_i \end{bmatrix}$$

Finite element discretization:

- for sufficiently large  $S$ , arbitrary  $P^k/P^l$  finite elements work ( $k, l = 1, 2, \dots$ );
- for small  $S$ , spurious pressure oscillations can appear  $\Rightarrow$  use Stokes-stable pair (e.g. Taylor-Hood  $P^{k+1}/P^k$  or MINI element)

## Approximation of two-field formulation of Biot system III



- in geosciences, usually lowest order approximations are used
- stress and flux are computed using solution gradient  
⇒ worse FE approximation
- remedy: mixed/dual formulations

# Approximation of three-field formulation of Biot system I

**Three-field (primal-dual) formulation**  $(\mathbf{u}, p, \mathbf{q})$ :

- additional unknown  $\mathbf{q}$  given by Darcy's law

$$\begin{aligned} -\operatorname{div}(\mathbf{C}\boldsymbol{\varepsilon}(\mathbf{u})) + \alpha\nabla p &= \mathbf{f} \\ \partial_t (\mathbf{S}p + \alpha \operatorname{div} \mathbf{u}) + \operatorname{div} \mathbf{q} &= b \\ k^{-1}\mathbf{q} + \nabla p &= \mathbf{0} \end{aligned}$$

Operator form of time-semidiscretized problem:

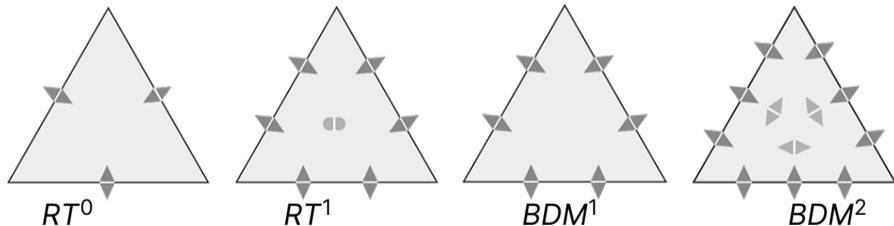
$$\begin{bmatrix} \mathcal{A} & -\mathcal{B}^\top & 0 \\ \mathcal{B} & \mathcal{C} & \mathcal{D} \\ 0 & -\mathcal{D}^\top & \mathcal{E} \end{bmatrix} \begin{bmatrix} \mathbf{u}(t_j) \\ p(t_j) \\ \mathbf{q}(t_j) \end{bmatrix} = \begin{bmatrix} \mathbf{f}_j \\ b_j \\ \mathbf{0} \end{bmatrix}$$

- two-fold skew-symmetric saddle-point structure

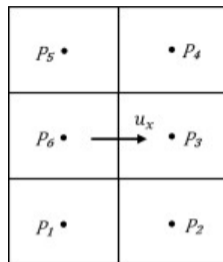
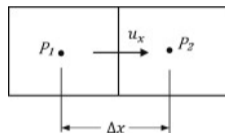
## Approximation of three-field formulation of Biot system II

FE/FV discretization:

- stable pressure-flux pair: e.g.  $P^k/RT^k$ ,  $P^k/BDM^{k+1}$ ,  $k = 0, 1, \dots$ , or finite volume methods  $P^0/TPFA$ ,  $P^0/MPFA$
- stable displacement-pressure pair: for lowest order pressure space,  $P^2/P^0$  works in 2D, in 3D more delicate issue



# Approximation of three-field formulation of Biot system III



Two-point / multi-point flux approximation FV schemes

# Approximation of five-field formulation of Biot system I

**Five-field (dual-dual) formulation**  $(\sigma, \mathbf{r}, \mathbf{u}, p, \mathbf{q})$ :

- additional unknown  $\sigma$  given by Hooke's law
- symmetry of  $\sigma$  enforced weakly...Lagrange multiplier  $\mathbf{r}$

$$-\operatorname{div} \sigma = \mathbf{f}$$

$$\partial_t (Sp + \alpha \operatorname{tr}(\mathbf{C}^{-1}(\sigma + \alpha p \mathbf{I}))) + \operatorname{div} \mathbf{q} = b$$

$$k^{-1} \mathbf{q} + \nabla p = \mathbf{0}$$

$$\mathbf{C}^{-1}(\sigma + \alpha p \mathbf{I}) - \nabla \mathbf{u} + as^* \mathbf{r} = \mathbf{0}$$

$$as \sigma = \mathbf{0}$$



## Approximation of five-field formulation of Biot system II

Operator form of time-semidiscretized problem:

$$\begin{bmatrix} \mathcal{A} & \mathcal{B}^\top & \mathcal{C}^\top & \mathcal{D}^\top & 0 \\ -\mathcal{B} & 0 & 0 & 0 & 0 \\ -\mathcal{C} & 0 & 0 & 0 & 0 \\ \mathcal{D} & 0 & 0 & \varepsilon & \mathcal{F}^\top \\ 0 & 0 & 0 & -\mathcal{F} & \mathcal{G} \end{bmatrix} \begin{bmatrix} \boldsymbol{\sigma}(t_j) \\ \mathbf{r}(t_j) \\ \mathbf{u}(t_j) \\ \rho(t_j) \\ \mathbf{q}(t_j) \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ \mathbf{f}_i \\ b_i \\ \mathbf{0} \end{bmatrix}$$

- block-symmetric saddle-point problem

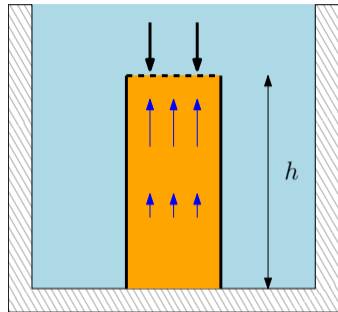
FE/FV discretization:

- stable mixed elasticity spaces: e.g.  $\boldsymbol{\sigma} \in \mathit{BDM}^1$ ,  $\mathbf{u} \in P^0$ ,  $\mathbf{r} \in P^0$  or MPSA/ $P^0/P^0$
- other variables similar as in previous case

# Simple problems I

## Terzaghi's 1D problem

- confined soil sample placed in container with liquid
- bottom side impermeable, top fully drained and subjected to constant vertical stress
- due to symmetry the problem can be solved in 1D
- analytical solution by Terzaghi (1923) in the form of infinite series
- applied stress induces sudden increase of pressure in the sample
- after consolidation, the pressure drops to the external level
- due to low permeability, consolidation takes certain time



## Simple problems II

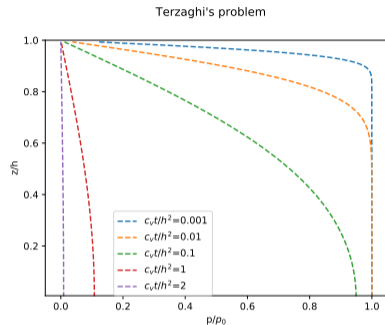
The 1D Biot problem can be reduced to a scalar parabolic equation

$$\partial_t p = c_v \partial_{xx}^2 p, \quad c_v = \frac{k}{S + \frac{\alpha^2}{K + \frac{4}{3}G}}$$

The quantity  $c_v t/h^2$  indicates whether the system is consolidated or not. In this problem, for

$$\frac{c_v t}{h^2} > 2$$

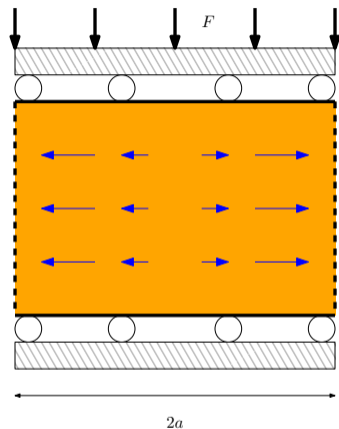
the pressure is almost constant.



## Simple problems III

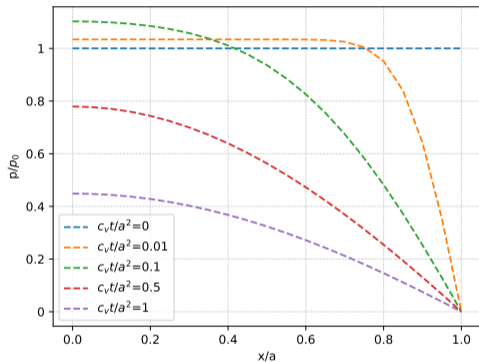
### Mandel's problem

- rectangular sample subjected to constant vertical stress
- lateral sides drained, top and bottom impermeable
- semi-analytical solution by Mandel (1963) in the form of infinite series, depending on roots of a nonlinear equation
- after consolidation period, pressure drops to exterior pressure
- due to low permeability and sudden pressure drop on lateral sides, pressure temporarily increases inside the domain (Mandel-Cryer effect)

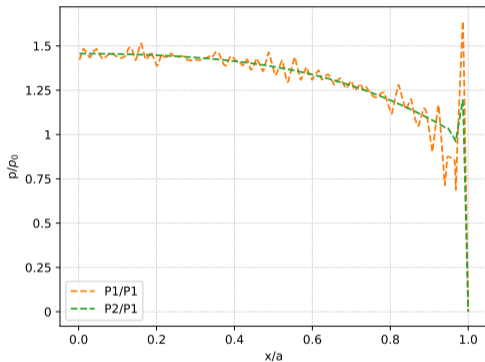


# Simple problems IV

Mandel's problem: analytical solution



Mandel's problem: FEM solution

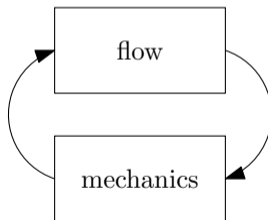


Left: Analytical solution of pressure, right: comparison of FEM solutions.

# Iterative splitting

## Iterative splittings

- Biot problem is fully coupled
- Monolithic solution is unconditionally stable, but large problems need suitable preconditioners
- Splitting of mechanics and flow can be advantageous but requires careful design



## Iterative splittings of skew-symmetric problems I

The discretized two- and three-field formulation of Biot system has skew-symmetric saddle-point structure:

$$\begin{bmatrix} \mathbf{A} & -\mathbf{B}^\top \\ \mathbf{B} & \mathbf{C} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1 \\ \mathbf{x}_2 \end{bmatrix} = \begin{bmatrix} \mathbf{f}_1 \\ \mathbf{f}_2 \end{bmatrix}$$

Block Gauss-Seidel method (BGS):

$$\begin{bmatrix} \mathbf{A} & -\mathbf{B}^\top \\ \mathbf{0} & \mathbf{C} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1^{i+1} \\ \mathbf{x}_2^{i+1} \end{bmatrix} + \begin{bmatrix} \mathbf{0} & \mathbf{0} \\ \mathbf{B} & \mathbf{0} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1^i \\ \mathbf{x}_2^i \end{bmatrix} = \begin{bmatrix} \mathbf{f}_1 \\ \mathbf{f}_2 \end{bmatrix}$$

It can be shown that BGS converges only if  $\mathbf{C} \succ \mathbf{BA}^{-1}\mathbf{B}^\top \Rightarrow$  conditional convergence.



## Iterative splittings of skew-symmetric problems II

This gives the **fixed-strain splitting**:

- 1 Given  $\mathbf{u}^i$ , find  $p^{i+1}$ :

$$\partial_t (Sp^{i+1} + \alpha \operatorname{div} \mathbf{u}^i) - \operatorname{div}(k \nabla p^{i+1}) = b$$

- 2 Given  $p^{i+1}$ , find  $\mathbf{u}^{i+1}$ :

$$-\operatorname{div}(\mathbf{C}\varepsilon(\mathbf{u}^{i+1})) + \alpha \nabla p^{i+1} = \mathbf{f}$$

Theorem (Conditional convergence of fixed-strain splitting)

*The fixed-strain splitting method is convergent under the condition*

$$S > \frac{\alpha^2}{K}.$$

## Iterative splittings of skew-symmetric problems III

Proof:

- differences  $(\delta_p^{i+1}, \delta_u^i) := (p^{i+1} - p^i, \mathbf{u}^i - \mathbf{u}^{i-1})$  satisfy:

$$\begin{aligned} \partial_t (\mathcal{S} \delta_p^{i+1} + \alpha \operatorname{div} \delta_u^i) - \operatorname{div}(k \nabla \delta p^{i+1}) &= 0 \\ -\operatorname{div}(\mathbf{C} \varepsilon(\delta_u^i)) + \alpha \nabla \delta p^i &= \mathbf{0} \end{aligned}$$

- multiply by  $\partial_t \delta_p^{i+1}$  / differentiate and multiply by  $\partial_t \delta_u^i$  and integrate:

$$\begin{aligned} S \left\| \partial_t \delta_p^{i+1} \right\|_2^2 + k \frac{d}{dt} \left\| \nabla \delta_p^{i+1} \right\|_2^2 + K \left\| \operatorname{div} \partial_t \delta_u^i \right\|_2^2 \\ + \underbrace{\alpha (\partial_t \delta_p^{i+1}, \operatorname{div} \partial_t \delta_u^i)} &\leq \underbrace{\alpha (\partial_t \delta_p^i, \operatorname{div} \partial_t \delta_u^i)} \\ \leq \frac{\alpha^2}{2K} \left\| \partial_t \delta_p^{i+1} \right\|_2^2 + \frac{K}{2} \left\| \operatorname{div} \partial_t \delta_u^i \right\|_2^2 &\leq \frac{\alpha^2}{2K} \left\| \partial_t \delta_p^i \right\|_2^2 + \frac{K}{2} \left\| \operatorname{div} \partial_t \delta_u^i \right\|_2^2 \end{aligned}$$

## Iterative splittings of skew-symmetric problems IV

- result:

$$\left(S - \frac{\alpha^2}{2K}\right) \left\| \partial_t \delta_p^{i+1} \right\|_2^2 + k \frac{d}{dt} \left\| \nabla \delta_p^{i+1} \right\|_2^2 \leq \frac{\alpha^2}{2K} \left\| \partial_t \delta_p^i \right\|_2^2$$

- convergence if

$$S - \frac{\alpha^2}{2K} > \frac{\alpha^2}{2K} \quad \Leftrightarrow \quad S > \frac{\alpha^2}{K}$$



## Iterative splittings of skew-symmetric problems V

Stabilized BGS: Schur complement approach

$$\begin{bmatrix} \mathbf{A} & -\mathbf{B}^\top \\ \mathbf{B} & \mathbf{C} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1 - \mathbf{x}_1^i \\ \mathbf{x}_2 - \mathbf{x}_2^i \end{bmatrix} = \begin{bmatrix} \mathbf{f}_1 \\ \mathbf{f}_2 \end{bmatrix} - \begin{bmatrix} \mathbf{A} & -\mathbf{B}^\top \\ \mathbf{B} & \mathbf{C} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1^i \\ \mathbf{x}_2^i \end{bmatrix}$$

Block LU factorization gives ( $\mathbf{S} = \mathbf{BA}^{-1}\mathbf{B}^\top$ ):

$$\begin{bmatrix} \mathbf{I} & \mathbf{0} \\ \mathbf{BA}^{-1} & \mathbf{I} \end{bmatrix} \begin{bmatrix} \mathbf{A} & -\mathbf{B}^\top \\ \mathbf{0} & \mathbf{C} + \mathbf{S} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1 - \mathbf{x}_1^i \\ \mathbf{x}_2 - \mathbf{x}_2^i \end{bmatrix} = \begin{bmatrix} \mathbf{f}_1 \\ \mathbf{f}_2 \end{bmatrix} - \begin{bmatrix} \mathbf{A} & -\mathbf{B}^\top \\ \mathbf{B} & \mathbf{C} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1^i \\ \mathbf{x}_2^i \end{bmatrix}$$

This leads to an iterative scheme with approximate Schur complement  $\tilde{\mathbf{S}}$ :

$$\begin{bmatrix} \mathbf{I} & \mathbf{0} \\ \mathbf{BA}^{-1} & \mathbf{I} \end{bmatrix} \begin{bmatrix} \mathbf{A} & -\mathbf{B}^\top \\ \mathbf{0} & \mathbf{C} + \tilde{\mathbf{S}} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1^{i+1} - \mathbf{x}_1^i \\ \mathbf{x}_2^{i+1} - \mathbf{x}_2^i \end{bmatrix} = \begin{bmatrix} \mathbf{f}_1 \\ \mathbf{f}_2 \end{bmatrix} - \begin{bmatrix} \mathbf{A} & -\mathbf{B}^\top \\ \mathbf{B} & \mathbf{C} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1^i \\ \mathbf{x}_2^i \end{bmatrix}$$

## Iterative splittings of skew-symmetric problems VI

Applying the inverse of the first matrix we get:

$$\begin{bmatrix} \mathbf{A} & -\mathbf{B}^\top \\ \mathbf{0} & \mathbf{C} + \tilde{\mathbf{S}} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1^{i+1} - \mathbf{x}_1^i \\ \mathbf{x}_2^{i+1} - \mathbf{x}_2^i \end{bmatrix} = \begin{bmatrix} \mathbf{f}_1 \\ \mathbf{f}_2 \end{bmatrix} - \begin{bmatrix} \mathbf{A} & -\mathbf{B}^\top \\ \mathbf{B} & \mathbf{C} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1^i \\ \mathbf{x}_2^i \end{bmatrix}$$

This can be rewritten as a stabilized BGS:

$$\begin{bmatrix} \mathbf{0} & \mathbf{0} \\ \mathbf{0} & \tilde{\mathbf{S}} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1^{i+1} - \mathbf{x}_1^i \\ \mathbf{x}_2^{i+1} - \mathbf{x}_2^i \end{bmatrix} + \begin{bmatrix} \mathbf{A} & -\mathbf{B}^\top \\ \mathbf{0} & \mathbf{C} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1^{i+1} \\ \mathbf{x}_2^{i+1} \end{bmatrix} + \begin{bmatrix} \mathbf{0} & \mathbf{0} \\ \mathbf{B} & \mathbf{0} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1^i \\ \mathbf{x}_2^i \end{bmatrix} = \begin{bmatrix} \mathbf{f}_1 \\ \mathbf{f}_2 \end{bmatrix}$$

or alternatively as a preconditioned Richardson method:

$$\begin{bmatrix} \mathbf{x}_1^{i+1} \\ \mathbf{x}_2^{i+1} \end{bmatrix} = \begin{bmatrix} \mathbf{x}_1^i \\ \mathbf{x}_2^i \end{bmatrix} + \begin{bmatrix} \mathbf{A} & -\mathbf{B}^\top \\ \mathbf{0} & \mathbf{C} + \tilde{\mathbf{S}} \end{bmatrix}^{-1} \left( \begin{bmatrix} \mathbf{f}_1 \\ \mathbf{f}_2 \end{bmatrix} - \begin{bmatrix} \mathbf{A} & -\mathbf{B}^\top \\ \mathbf{B} & \mathbf{C} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1^i \\ \mathbf{x}_2^i \end{bmatrix} \right)$$

# Iterative splittings of skew-symmetric problems VII

Application on two-field Biot system:

- Approximate Schur complement: scaled  $L^2$ -product  $\beta(p, q)$
- Result: **Fixed-stress splitting**

① Given  $(\mathbf{u}^i, p^i)$ , find  $p^{i+1}$ :

$$\beta \partial_t (p^{i+1} - p^i) + \partial_t (Sp^{i+1} + \alpha \operatorname{div} \mathbf{u}^i) - \operatorname{div}(k \nabla p^{i+1}) = b$$

② Given  $p^{i+1}$ , find  $\mathbf{u}^{i+1}$ :

$$-\operatorname{div}(\mathbf{C} \varepsilon(\mathbf{u}^{i+1})) + \alpha \nabla p^{i+1} = \mathbf{f}$$

Theorem (Unconditional convergence of fixed-stress splitting)

*The fixed-stress splitting method is convergent if  $\beta \geq \frac{1}{2} \frac{\alpha^2}{K}$ . Fastest convergence is obtained for  $\beta = \frac{1}{2} \frac{\alpha^2}{K}$ .*

## Iterative splittings of skew-symmetric problems VIII

Proof:

- differences  $(\delta_p^{i+1}, \delta_u^i)$  satisfy:

$$(S + \beta) \partial_t \delta_p^{i+1} - \operatorname{div}(k \nabla \delta p^{i+1}) + \alpha \operatorname{div} \partial_t \delta_u^i = \beta \partial_t \delta_p^i \\ - \operatorname{div}(\mathbf{C} \varepsilon(\delta_u^i)) + \alpha \nabla \delta p^i = \mathbf{0}$$

- multiply by  $\partial_t \delta_p^{i+1}$  / differentiate and multiply by  $\partial_t \delta_u^i$  and integrate:

$$(S + \beta) \left\| \partial_t \delta_p^{i+1} \right\|_2^2 + k \frac{d}{dt} \left\| \nabla \delta p^{i+1} \right\|_2^2 + K \left\| \operatorname{div} \partial_t \delta_u^i \right\|_2^2 \\ + \alpha (\partial_t \delta_p^{i+1}, \operatorname{div} \partial_t \delta_u^i) \leq (\partial_t \delta_p^i, \underbrace{\beta \partial_t \delta_p^{i+1} + \alpha \operatorname{div} \partial_t \delta_u^i}_{=: \sigma}) \\ \leq \frac{\beta}{2} \left\| \partial_t \delta_p^i \right\|_2^2 + \frac{1}{2\beta} \left\| \sigma \right\|_2^2$$

## Iterative splittings of skew-symmetric problems IX

- polarization identity:

$$\alpha(\partial_t \delta_p^{j+1}, \operatorname{div} \partial_t \delta_u^j) = \frac{1}{2\beta} \|\sigma\|_2^2 - \frac{\beta}{2} \|\partial_t \delta_p^{j+1}\|_2^2 - \frac{\alpha^2}{2\beta} \|\operatorname{div} \partial_t \delta_u^j\|_2^2$$

- result:

$$\begin{aligned} \left(S + \frac{\beta}{2}\right) \|\partial_t \delta_p^{j+1}\|_2^2 + k \frac{d}{dt} \|\nabla \delta_p^{j+1}\|_2^2 + \frac{1}{2\beta} \|\sigma\|_2^2 + \left(K - \frac{\alpha^2}{2\beta}\right) \|\operatorname{div} \partial_t \delta_u^j\|_2^2 \\ \leq \frac{\beta}{2} \|\partial_t \delta_p^j\|_2^2 + \frac{1}{2\beta} \|\sigma\|_2^2 \end{aligned}$$

- convergence if

$$K - \frac{\alpha^2}{2\beta} \geq 0 \quad \Leftrightarrow \quad \beta \geq \frac{\alpha^2}{2K}$$



# Iterative splittings of skew-symmetric problems X

One can also switch the order of elasticity and flow

- Approximate Schur complement: scaled divergence  $\gamma \operatorname{div}(\mathbf{u}^{i+1} - \mathbf{u}^i)$
- Resulting scheme: **undrained splitting**

① Given  $(\mathbf{u}^i, p^i)$ , find  $\mathbf{u}^{i+1}$ :

$$-\operatorname{div}(\gamma \operatorname{div}(\mathbf{u}^{i+1} - \mathbf{u}^i) \mathbf{I} + \mathbf{C} \boldsymbol{\varepsilon}(\mathbf{u}^{i+1})) + \alpha \nabla p^i = \mathbf{f}$$

② Given  $\mathbf{u}^{i+1}$ , find  $p^{i+1}$ :

$$\partial_t (S p^{i+1} + \alpha \operatorname{div} \mathbf{u}^{i+1}) - \operatorname{div}(k \nabla p^{i+1}) = b$$

Theorem (Unconditional convergence of undrained splitting)

*The undrained splitting method is convergent for  $\gamma \geq \frac{\alpha^2}{2S}$ .*

# Iterative splittings of skew-symmetric problems XI

## Remarks:

- Stable schemes: fixed-stress and undrained split
- Fixed-stress split generally has faster convergence
- Undrained split adds constraints on discretization of elasticity to avoid spurious oscillations
- References: Mikelić and Wheeler (2013), White et al. (2016), Both et al. (2017)

## Iterative splitting of symmetric problems I

The discretized five-field formulation of Biot system has symmetric saddle-point structure:

$$\begin{bmatrix} \mathbf{A} & \mathbf{B}^\top \\ \mathbf{B} & \mathbf{C} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1 \\ \mathbf{x}_2 \end{bmatrix} = \begin{bmatrix} \mathbf{f}_1 \\ \mathbf{f}_2 \end{bmatrix}$$

Since the matrix is blockwise s.p.d., the BGS is equivalent to alternating minimization

$$\mathbf{x}_1^i \rightarrow \mathbf{x}_2^i := \underset{\mathbf{y}}{\operatorname{argmin}} J(\mathbf{x}_1^i, \mathbf{y}), \quad \mathbf{x}_2^i \rightarrow \mathbf{x}_1^{i+1} := \underset{\mathbf{y}}{\operatorname{argmin}} J(\mathbf{y}, \mathbf{x}_2^i)$$

of the quadratic functional

$$J(\mathbf{x}_1, \mathbf{x}_2) = \left( \frac{1}{2} \begin{bmatrix} \mathbf{A} & \mathbf{B}^\top \\ \mathbf{B} & \mathbf{C} \end{bmatrix} \begin{bmatrix} \mathbf{x}_1 \\ \mathbf{x}_2 \end{bmatrix} - \begin{bmatrix} \mathbf{f}_1 \\ \mathbf{f}_2 \end{bmatrix} \right) \cdot \begin{bmatrix} \mathbf{x}_1 \\ \mathbf{x}_2 \end{bmatrix}$$

This method is unconditionally stable.

## Iterative splitting of symmetric problems II

Resulting scheme: **fixed-stress splitting**

① Given  $\sigma^i$ , find  $(p^{i+1}, \mathbf{q}^{i+1})$ :

$$\partial_t (S p^{i+1} + \alpha \operatorname{tr}(\mathbf{C}^{-1}(\sigma^i + \alpha p^{i+1} \mathbf{I}))) + \operatorname{div} \mathbf{q}^{i+1} = b$$

$$k^{-1} \mathbf{q}^{i+1} + \nabla p^{i+1} = \mathbf{0}$$

② Given  $p^{i+1}$ , find  $(\sigma^{i+1}, \mathbf{r}^{i+1}, \mathbf{u}^{i+1})$ :

$$-\operatorname{div} \sigma^{i+1} = \mathbf{f}$$

$$\mathbf{C}^{-1}(\sigma^{i+1} + \alpha p^{i+1} \mathbf{I}) - \nabla \mathbf{u}^{i+1} + \operatorname{as}^* \mathbf{r}^{i+1} = \mathbf{0}$$

$$\operatorname{as} \sigma^{i+1} = \mathbf{0}$$

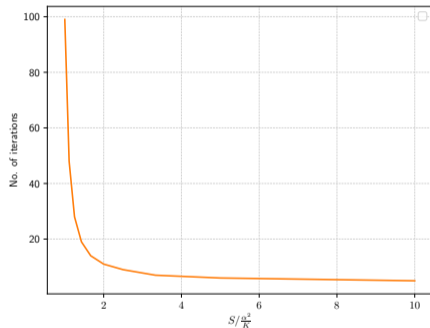
### Theorem

*The fixed-stress splitting method for five-field formulation is convergent.*

# Convergence of iterative schemes: example

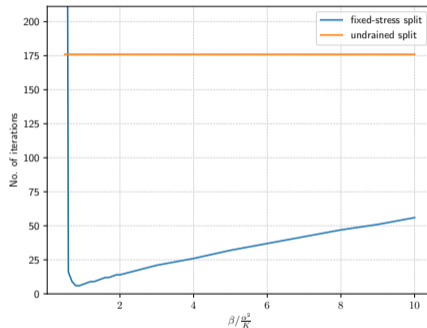
## Mandel's problem, implicit Euler, P2/P1 FEM

Mandel's problem: Convergence of fixed-strain splitting



Conditional convergence of fixed-strain splitting.

Mandel's problem: Convergence of fixed-stress/undrained splitting



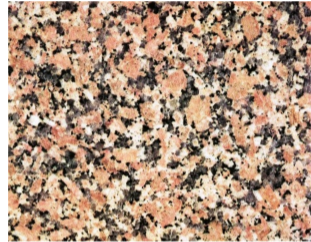
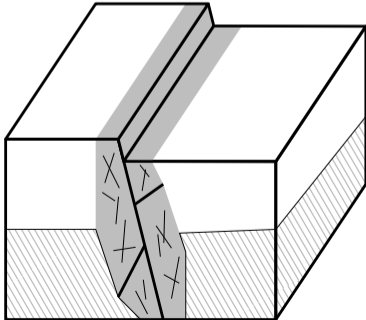
Convergence of fixed-stress splitting and undrained splitting.

## Applications in rock hydro-mechanics

# Modelling heterogeneities in rock hydro-mechanics I

Two types of rock heterogeneity:

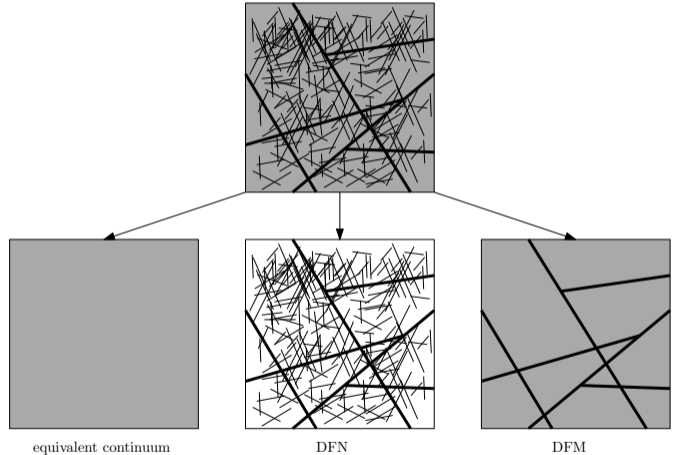
- local variations in bulk properties
- macroscopic fractures and fault zones with narrow width but large size



# Modelling heterogeneities in rock hydro-mechanics II

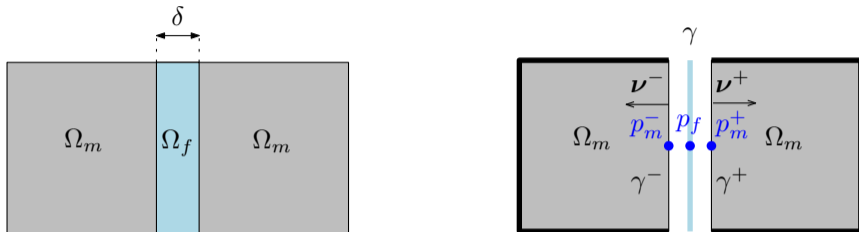
Modelling approaches:

- equivalent continuum
- discrete fracture network (DFN)
- discrete fracture-matrix (DFM)





# Discrete fracture-matrix models



- fields  $(\mathbf{u}, p, \dots)$  in  $\Omega_m \cup \Omega_f \rightarrow$  fields  $(\mathbf{u}_m, p_m, \dots)$  in  $\Omega_m$  and  $(\mathbf{u}_f, p_f, \dots)$  in  $\gamma$
- semi-discrete operators:

$$\tilde{\nabla} p = \nabla_t p_f + \nabla_v p, \quad \nabla_v p = \frac{1}{2} (\Delta^+ p \mathbf{v}^+ + \Delta^- p \mathbf{v}^-), \quad \Delta^\pm p = \frac{2}{\delta} (p_m^\pm - p_f)$$

$$\tilde{\text{div}} \mathbf{u} = \text{div}_t \mathbf{u}_f + \text{div}_v \mathbf{u}, \quad \text{div}_v \mathbf{u} = \frac{1}{2} (\Delta^+ \mathbf{u} \cdot \mathbf{v}^+ + \Delta^- \mathbf{u} \cdot \mathbf{v}^-)$$

# DFM model of poroelasticity

- ① Biot equations in rock matrix  $\Omega_m$ :

$$\begin{aligned} -\operatorname{div} \boldsymbol{\sigma}_m + \alpha \nabla p_m &= \mathbf{f}_m, & \boldsymbol{\sigma}_m &= \mathbf{C} \boldsymbol{\varepsilon}(\mathbf{u}_m) \\ \partial_t (S p_m + \alpha \operatorname{div} \mathbf{u}_m) + \operatorname{div} \mathbf{q}_m &= b_m, & \mathbf{q}_m &= k \nabla p_m \end{aligned}$$

- ② Biot equations in fracture  $\gamma$ :

$$\begin{aligned} -\widetilde{\operatorname{div}} \boldsymbol{\sigma} + \alpha \widetilde{\nabla} p &= \mathbf{f}_f, & \boldsymbol{\sigma}_f &= \frac{1}{2} \mathbf{C} (\widetilde{\nabla} \mathbf{u} + \widetilde{\nabla} \mathbf{u}^T) \\ \partial_t (S p_f + \alpha \widetilde{\operatorname{div}} \mathbf{u}) + \widetilde{\operatorname{div}} \mathbf{q} &= b_f, & \mathbf{q}_f &= k \widetilde{\nabla} p \end{aligned}$$

- ③ Continuity of flux and tangential traction on fracture-matrix interface:

$$\mathbf{q}_m^\pm \cdot \mathbf{v}^\pm = \mathbf{q}_f^\pm \cdot \mathbf{v}^\pm \quad (\boldsymbol{\sigma}_m^\pm \mathbf{v}^\pm)_t = (\boldsymbol{\sigma}_f^\pm \mathbf{v}^\pm)_t$$

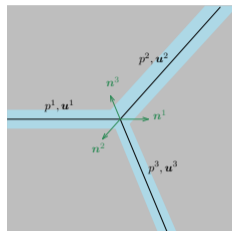
- ④ Constraints on minimal fracture aperture:

$$\delta + (\mathbf{u}_m^+ \cdot \mathbf{v}^+ + \mathbf{u}_m^- \cdot \mathbf{v}^-) \geq \delta_{min} \quad (\Delta^+ \boldsymbol{\sigma} + \Delta^- \boldsymbol{\sigma}) \mathbf{v} \cdot \mathbf{v} \geq 0$$

# Discretization of DFM model I

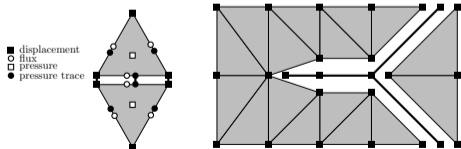
- Generalization to immersed fractures, crossings and branching:

$$\sum_i \sigma_f^i \mathbf{n}^i = \mathbf{0}$$
$$\sum_i \mathbf{q}_f^i \cdot \mathbf{n}^i = 0$$



- Compatible discretization of domain and fracture
- FE spaces: P1/MH

displacement	$P_1$
pressure	$P_0$
flux	$RT_0$
pressure trace	$P_0$ on edges



## Discretization of DFM model II

- algebraic form of fixed-stress splitting scheme:

$$\min_{\mathbf{u}_i^m} \left( \frac{1}{2} \mathbf{A} \mathbf{u}_i^m - \mathbf{f}^m - \mathbf{B}^\top \mathbf{u}_i^f \right) \cdot \mathbf{u}_i^m, \quad \mathbf{E} \mathbf{u}_i^m \leq \mathbf{c}$$

$$(\mathbf{C} + \beta \tilde{\mathbf{S}}) \mathbf{u}_{i+1}^f = \mathbf{f}^f + \beta \tilde{\mathbf{S}} \mathbf{u}_i^f - \mathbf{B} \mathbf{u}_i^m$$

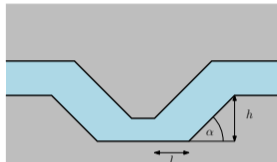
- contact problems solved using quadratic programming (PERMON)
- implementation: Flow123d



Joint work with J. Kružík, D. Horák

# Nonlinear fracture couplings

- Contact with shear dilation



$$\delta_{min} = \delta_{min}(\mathbf{u}) \dots \text{bounded, Lipschitz continuous}$$

Solution by successive approximations:

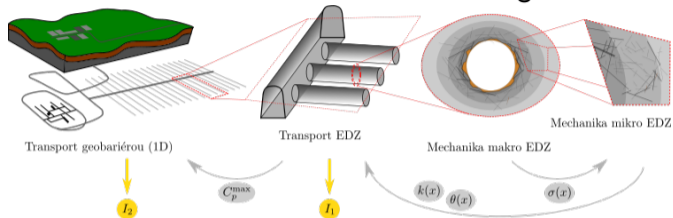
$$\mathcal{P} : \delta_{min} \mapsto \mathbf{u}; \quad \mathbf{u}^{n+1} := \mathcal{P}(\delta_{min}(\mathbf{u}^n))$$

- Cubic law for fracture permeability

$$k_f = \frac{(\delta + \mathbf{u}^+ \cdot \mathbf{v}^+ + \mathbf{u}^- \cdot \mathbf{v}^-)^2}{12\mu}$$

# Application: Homogenization of conductivity in EDZ I

## Multiscale model of excavation damage zone

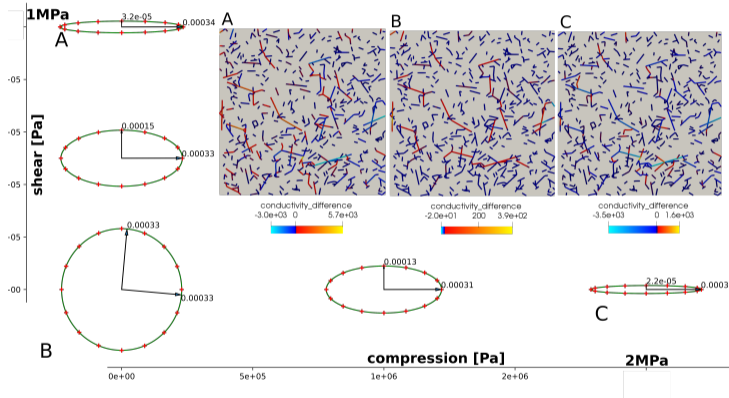


Computation of equivalent hydraulic conductivity of a local DFM model:

- series of computations with prescribed pressure gradient
- least squares fitting of conductivity tensor from averaged velocity and given pressure gradient
- influence of stress/deformation

Joint work with J. Březina, M. Špetlík

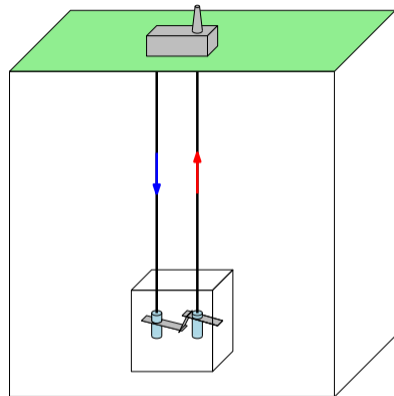
# Application: Homogenization of conductivity in EDZ II



## Application: Geothermal system I

- Thermo-hydraulic model of geothermal heat exchanger
- Hydro-mechanical model to describe stimulation (opening) of preexisting fractures
- Fractures with high permeability and low stiffness
- Simulation of power during 30 years of operation
- Comparison for
  - no stimulation
  - stimulation by nonlinear HM model with fracture contact and cubic law

Joint work with J. Březina, P. Exner

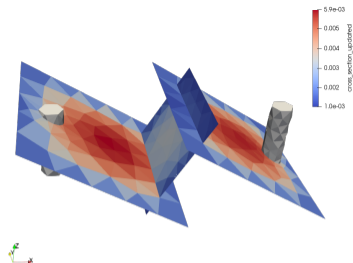


rock: granite  
depth: 5 km  
distance of wells: 200 m  
open part of wells: 100 m  
computational domain: cube 600 m \\  
cylinders  $\varnothing = 10$  m around wells

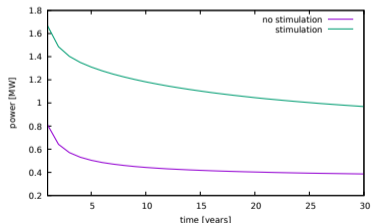


# Application: Geothermal system II

- Fractures:  
initial cross-section: 1 mm  
initial conductivity:  $k_f/k_m = 10^5$   
Young modulus:  $E_f/E_m = 10^{-9}$
- ① **HM model of hydraulic stimulation**  
injection pressure: 10 MPa  
initial cross-section: 1 mm
- ② **TH model of heat production**  
injection pressure: 1 MPa  
bottom temperature: 150°C  
input water temperature: 15°C

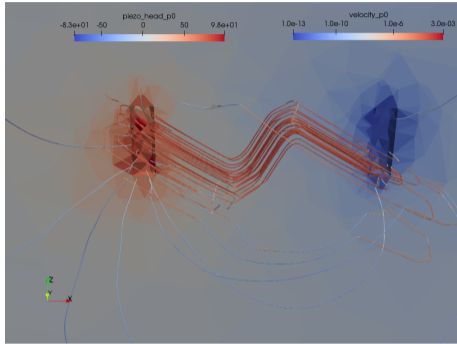


Cross-section of stimulated fractures

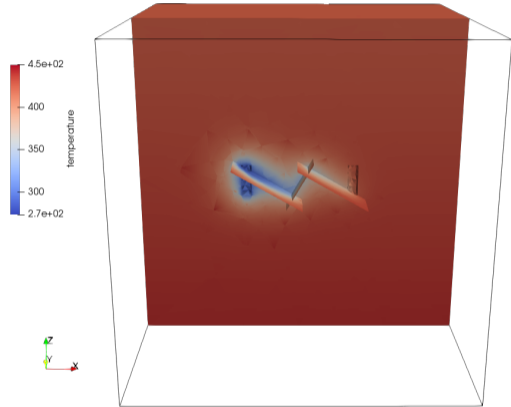


Power (with and without stimulation)

# Application: Geothermal system III



Piezometric head and velocity streamlines



Temperature after 30 years

# Conclusion

## Biot system of poroelasticity

- equivalent formulations using primal/dual variables
- FEM/FVM approximations and their stability
- iterative splittings and their stability
- DFM models for fractured rocks
- real-world applications

FACULTY OF MECHATRONICS,  
INFORMATICS AND INTERDISCIPLINARY  
STUDIES TUL



**Thank you for attention!**

**Jan Stebel**

Technical University of Liberec, Czech Republic

jan.stebel@tul.cz

## References: Modelling of poroelasticity I

M.A. Biot. General theory of three-dimensional consolidation, J. Appl. Phys., 12, 155-164, 1941.

E. Detournay and A.H.-D. Cheng. Fundamentals of poroelasticity, Comprehensive Rock Engineering: Principles, Practice and Projects, Vol. II, Analysis and Design Method, C. Fairhurst (editor), Pergamon Press, 113-171, 1993.

F. Gassmann. Über die Elastizität poröser Medien, Vierteljahrsschrift der Naturforschenden Gesellschaft in Zürich, 96, 1-23, 1951.

K. Terzaghi. Die Berechnung der Durchlässigkeitsziffer des Tones aus dem Verlauf der hydrodynamische Spannungserscheinungen, Sitzber. Akad. Wiss. Wien, Abt. IIa, 132, 125-138, 1923.

## References: Modelling of poroelasticity II

K. Terzaghi. Erdbaumechanik auf bodenphysikalischer Grundlage, Deuticke, Wien, 1925.

A. Verruijt. Theory and problems of poroelasticity, 2016. On-line resource, accessible at <https://geo.verruijt.net/software/PoroElasticity2016b.pdf>.

## References: Well-posedness

R. E. Schowalter. Diffusion in poro-elastic media. *Journal of Mathematical Analysis and Applications*, 251(1):310–340, 2000.

A. Ženíšek. The existence and uniqueness theorem in Biot's consolidation theory. *Aplikace Matematiky*, 29(3):194–211, 1984.

## References: Iterative splittings

J. W. Both, M. Borregales, J. M. Nordbotten, K. Kumar, F. A. Radu. Robust fixed stress splitting for Biot's equations in heterogeneous media. *Applied Mathematics Letters* 68:101–108, 2017.

A. Mikelić and M. F. Wheeler. Convergence of iterative coupling for coupled flow and geomechanics. *Computational Geosciences* 17:455–461, 2013.

J. A. White, N. Castelletto, H. A. Tchelepi. Block-partitioned solvers for coupled poromechanics: A unified framework. *Comput. Methods Appl. Mech. Engrg.* 303:55–74, 2016.